

# **European technical assessment**

# MKT Injection system VMU Plus for concrete

valid for

# **Injection anchor XV Plus**

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Approval body for construction products and types of construction

#### **Bautechnisches Prüfamt**

An institution established by the Federal and Laender Governments



# European Technical Assessment

## ETA-11/0415 of 13 November 2015

English translation prepared by DIBt - Original version in German language

#### **General Part**

Technical Assessment Body issuing the Deutsches Institut für Bautechnik **European Technical Assessment:** Trade name of the construction product Injection System VMU plus for concrete Product family Bonded Anchor for use in concrete to which the construction product belongs MKT Manufacturer Metall-Kunststoff-Technik GmbH & Co. KG Auf dem Immel 2 67685 Weilerbach DEUTSCHLAND MKT Manufacturing plant Metall-Kunststoff-Technik GmbH & Co. KG Auf dem Immel 2 67685 Weilerbach DEUTSCHLAND This European Technical Assessment 24 pages including 3 annexes which form an integral part of this assessment contains This European Technical Assessment is Guideline for European technical approval of "Metal issued in accordance with Regulation (EU) anchors for use in concrete", ETAG 001 Part 5: "Bonded No 305/2011, on the basis of anchors", April 2013, used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011.

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## European Technical Assessment ETA-11/0415

Page 2 of 24 | 13 November 2015

English translation prepared by DIBt

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Page 3 of 24 | 13 November 2015

#### Specific Part

#### 1 Technical description of the product

The "Injection system VMU plus for concrete" is a bonded anchor consisting of a cartridge with injection mortar VMU plus or VMU plus Polar and a steel element. The steel element consist of a commercial threaded rod with washer and hexagon nut in the range of M8 to M30 or a reinforcing bar in the range of diameter 8 to 32 mm.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between metal part, injection mortar and concrete.

The product description is given in Annex A.

#### 2 Specification of the intended use in accordance with the applicable European Assessment Document

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annex B.

The verifications and assessment methods on which this European Technical Assessment is based lead to the assumption of a working life of the anchor of at least 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

#### 3 Performance of the product and references to the methods used for its assessment

#### 3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance tension and shear loads	See Annex C 1 to C 8
Displacements under tension and shear loads	See Annex C 9 / C 10

#### 3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance
Reaction to fire	Anchorages satisfy requirements for Class A1
Resistance to fire	No performance assessed

#### 3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances there may be requirements (e.g. transposed European legislation and national laws, regulations and administrative provisions) applicable to the products falling within the scope of this European Technical Assessment. In order to meet the provisions of Regulation (EU) No 305/2011, these requirements need also to be complied with, when and where they apply.

#### 3.4 Safety in use (BWR 4)

The essential characteristics regarding Safety in use are included under the Basic Works Requirement Mechanical resistance and stability.



# European Technical Assessment ETA-11/0415

Page 4 of 24 | 13 November 2015

English translation prepared by DIBt

# 4 Assessment and verification of constancy of performance (AVCP) system applied, with reference to its legal base

In accordance with guideline for European technical approval ETAG 001, April 2013 used as European Assessment Document (EAD) according to Article 66 Paragraph 3 of Regulation (EU) No 305/2011 the applicable European legal act is: [96/582/EC].

The system to be applied is: 1

# 5 Technical details necessary for the implementation of the AVCP system, as provided for in the applicable EAD

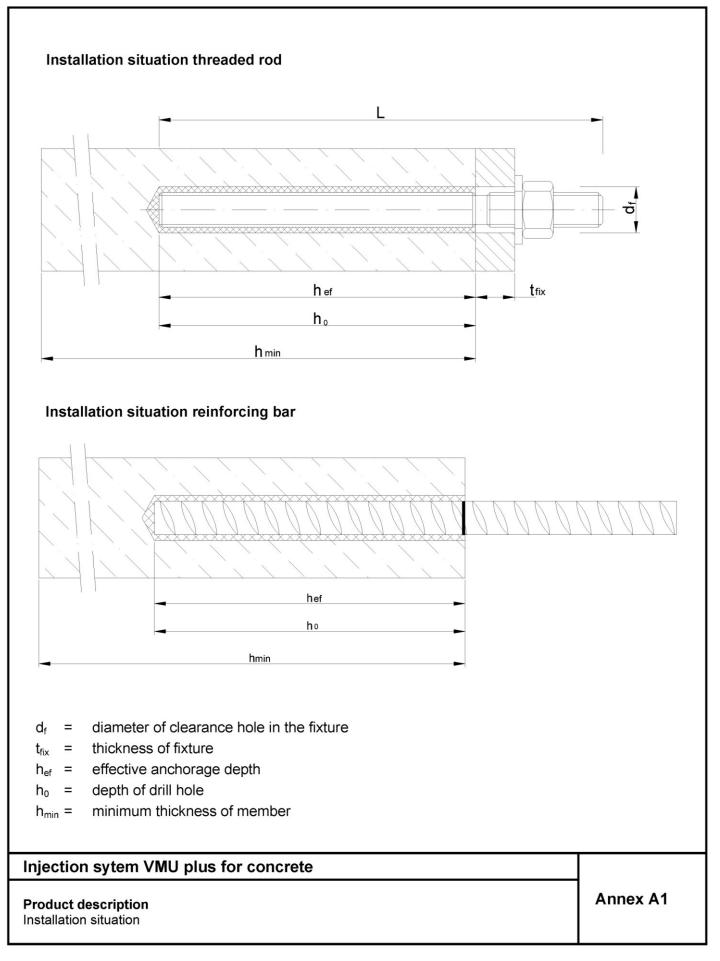
Technical details necessary for the implementation of the AVCP system are laid down in the control plan deposited with Deutsches Institut für Bautechnik.

Issued in Berlin on 13 November 2015 by Deutsches Institut für Bautechnik

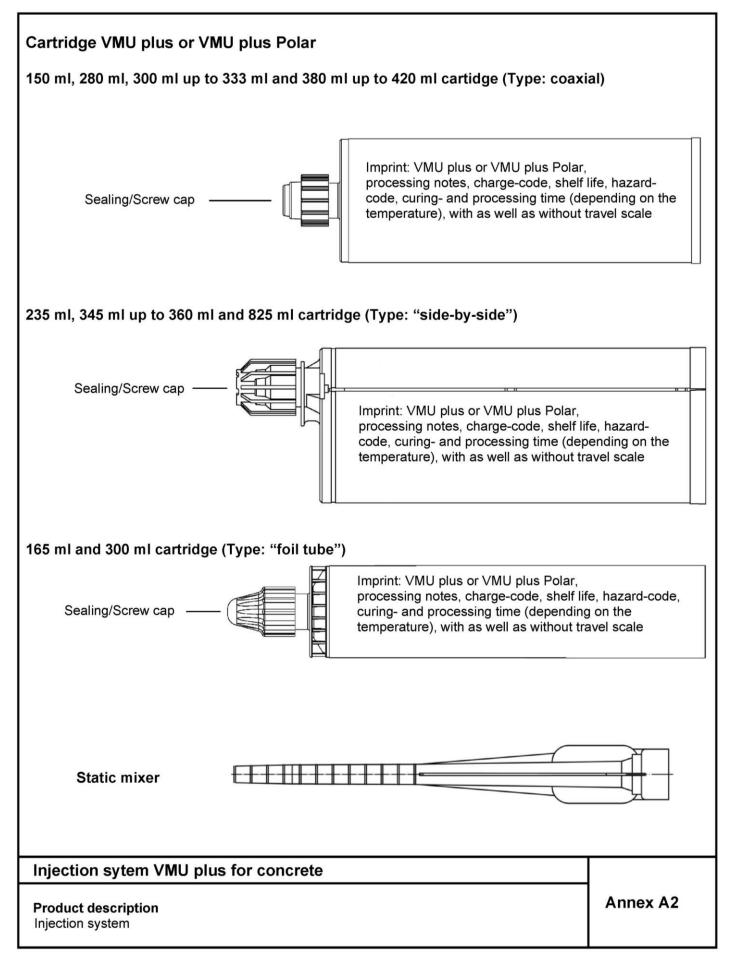
Uwe Bender Head of Department *beglaubigt:* G. Lange

# Page 5 of European Technical Assessment ETA-11/0415 of 13 November 2015

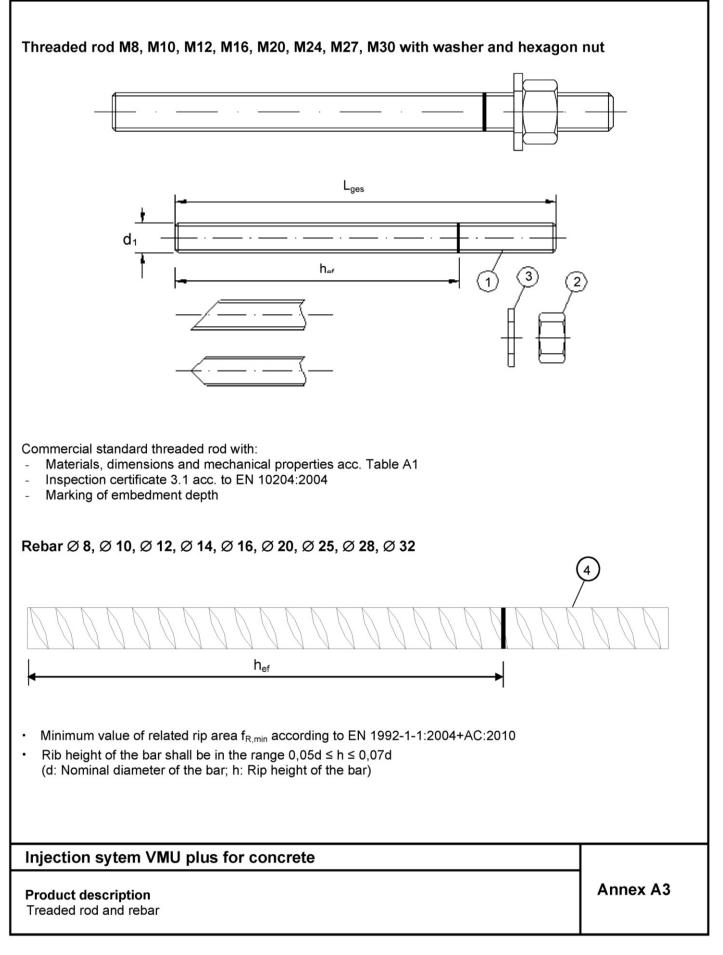














Part	Part Designation Material								
	, zinc plated ≥ 5 μm acc. to EN ISO 404 , hot-dip galvanised ≥ 40 μm acc. to EN	2:1999 or I ISO 1461:2009 and EN ISO 10684:2004+AC::	2009						
1	Anchor rod	Steel, EN 10087:1998 or EN 10263:2001 Property class 4.6, 4.8, 5.8, 8.8, EN 1993-1-8:2 A <sub>5</sub> > 8 % fracture elongation	2005+AC:2009						
2	Hexagon nut	Steel acc. to EN 10087:1998 or EN 10263:200 Property class 4 (for class 4.6 or 4.8 rod) EN IS Property class 5 (for class 5.8 rod) EN ISO 898 Property class 8 (for class 8.8 rod) EN ISO 898	SO 898-2:2012, 3-2:2012,						
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Steel, zinc plated or hot-dip galvanised							
Stain	less steel								
1	Anchor rod	Material 1.4401 / 1.4404 / 1.4571 / 1.4362, EN > M24: Property class 50 EN ISO 3506-1:2009 ≤ M24: Property class 70 EN ISO 3506-1:2009 A₅ > 8% fracture elongation							
2         Material 1.4401 / 1.4404 / 1.4571 /1.4362, EN 10088:2005,           2         Hexagon nut         > M24: Property class 50 (for class 50 rod) EN ISO 3506-2:2009           ≤         M24: Property class 70 (for class 70 rod) EN ISO 3506-2:2009									
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4401, 1.4404, 1.4571 or 1.4362, EN							
High	corrosion resistant steel								
1	Anchor rod	Material 1.4529 / 1.4565, EN 10088-1:2005, > M24: Property class 50 EN ISO 3506-1:2009 ≤ M24: Property class 70 EN ISO 3506-1:2009 A₅ > 8% fracture elongation							
2	2         Hexagon nut         Material 1.4529 / 1.4565 EN 10088-1:2005, > M24: Property class 50 (for class 50 rod) EN ISO 3506-2:2009 ≤ M24: Property class 70 (for class 70 rod) EN ISO 3506-2:2009								
3	Washer, EN ISO 887:2006, EN ISO 7089:2000, EN ISO 7093:2000 or EN ISO 7094:2000	Material 1.4529 / 1.4565, EN 10088-1:2005							
Reba	r								
4	Rebar EN 1992-1-1:2004+AC:2010, Annex C	Bars and de-coiled rods class B or C $f_{yk}$ and k according to NDP or NCL of EN 1992- $f_{uk} = f_{tk} = k \cdot f_{yk}$	-1-1/NA:2013						
Injec	ction sytem VMU plus for concre	ete							
<b>Prod</b> Mater	uct description rials		Annex A4						



#### Specification of intended use

#### Anchorages subject to:

- Static and quasi-static loads: M8 to M30, Rebar Ø8 to Ø32.
- Seismic action for Performance Category C1: M8 to M30, Rebar Ø8 to Ø32.

#### **Base materials:**

- Reinforced or unreinforced normal weight concrete according to EN 206-1:2000.
- Strength classes C20/25 to C50/60 according to EN 206-1:2000.
- Cracked and non-cracked concrete: M8 to M30, Rebar Ø8 to Ø32.

#### **Temperature Range:**

- I: 40 °C to +40 °C (max long term temperature +24 °C and max short term temperature +40 °C)
- II: 40 °C to +80 °C (max long term temperature +50 °C and max short term temperature +80 °C)
- III: 40 °C to +120 °C (max long term temperature +72 °C and max short term temperature +120 °C)

#### Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure (including industrial and marine environment) and to
  permanently damp internal condition, if no particular aggressive conditions exist
  (stainless steel or high corrosion resistant steel).
- Structures subject to external atmospheric exposure and to permanently damp internal condition, if other particular aggressive conditions exist (high corrosion resistant steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

#### Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position
  of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to
  supports, etc.).
- · Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work.
- Anchorages under static or guasi-static actions are designed in accordance with:
- EOTA Technical Report TR 029 "Design of bonded anchors", Edition September 2010 or
  - CEN/TS 1992-4:2009
- Anchorages under seismic actions (cracked concrete) are designed in accordance with:
  - EOTA Technical Report TR 045 "Design of Metal Anchors under Seismic Action", Edition February 2013
  - Anchorages shall be positioned outside of critical regions (e.g. plastic hinges) of the concrete structure.
  - Fastenings in stand-off installation or with a grout layer are not allowed.

#### Installation:

- Dry or wet concrete: M8 to M30, Rebar Ø8 to Ø32.
- Flooded holes (not sea water): M8 to M16, Rebar Ø8 to Ø16.
- · Hole drilling by hammer or compressed air drill mode.
- · Overhead installation allowed.
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

### Injection sytem VMU plus for concrete

Intended Use Specifications



Table B1: Installation	n param	eters	for thre	eaded r	od					
Threaded rod			M8	M10	M12	M16	M20	M24	M27	M30
Nominal drill hole diameter	d <sub>0</sub> =	[mm]	10	12	14	18	24	28	32	35
Effective anchorage depth	h <sub>ef,min</sub> =	[mm]	60	60	70	80	90	96	108	120
Ellective anchorage depth	h <sub>ef,max</sub> =	[mm]	160	200	240	320	400	480	540	600
Diameter of clearance hole in the fixture	d <sub>f</sub> ≤	[mm]	9	12	14	18	22	26	30	33
Diameter of steel brush	d <sub>b</sub>	[mm]	12	14	16	20	26	30	34	37
Torque moment	T <sub>inst</sub> ≤	[mm]	10	20	40	80	120	160	180	200
Thickness of fixture	t <sub>fix,min</sub> >	[mm]				(	D			
Thickness of fixture	t <sub>fix,max</sub> <	[mm]				15	00			
Minimum thickness of member	h <sub>min</sub>	[mm]	1000	<sub>∍f</sub> + 30 m ≥ 100 mn				$h_{ef} + 2d_0$		
Minimum spacing	S <sub>min</sub>	[mm]	40	50	60	80	100	120	135	150
Minimum edge distance	C <sub>min</sub>	[mm]	40	50	60	80	100	120	135	150

#### Installation parameters for rebar Table B2:

Rebar	Rebar				Ø 12	Ø 14	Ø 16	Ø <b>20</b>	Ø 25	Ø 28	Ø 32
Nominal drill hole diameter	d <sub>0</sub> =	[mm]	12	14	16	18	20	24	32	35	40
Effective anchorage	h <sub>ef,min</sub> =	[mm]	60	60	70	75	80	90	100	112	128
depth	h <sub>ef,max</sub> =	[mm]	160	200	240	280	320	400	480	540	640
Diameter of steel brush	d <sub>b</sub>	[mm]	14	16	18	20	22	26	34	37	41,5
Minimum thickness of member	h <sub>min</sub>	[mm]		30 mm 0 mm				h <sub>ef</sub> + 2d <sub>0</sub>	)		
Minimum spacing	S <sub>min</sub>	[mm]	40	50	60	70	80	100	125	140	160
Minimum edge distance	C <sub>min</sub>	[mm]	40	50	60	70	80	100	125	140	160

# Injection sytem VMU plus for concrete

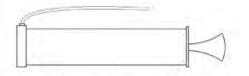
Intended Use Installation parameters



# Steel brush

## Table B3: Parameter cleaning and setting tools

Retaining wash	d <sub>b,min</sub> min. Brush - Ø	d₅ Brush - Ø	d₀ Drill bit - Ø	Rebar	Threaded Rod R	
[-]	[mm]	[mm]	[mm]	[mm]	[mm]	
1	10,5	12	10		M8	
	12,5	14	12	8	M10	
No	14,5	16	14	10	M12	
<ul> <li>Retaining wash required</li> </ul>	16,5	18	16	12		
	18,5	20	18	14	M16	
	20,5	22	20	16		
VM-IA 24	24,5	26	24	20	M20	
VM-IA 28	28,5	30	28		M24	
VM-IA 32	32,5	34	32	25	M27	
VM-IA 35	35,5	37	35	28	M30	
VM-IA 40	40,5	41,5	40	32		



P2-C

Recommended compressed air tool (min 6 bar) All applications

**Blow-out pump (volume 750ml)** Drill bit diameter  $(d_0)$ : 10 mm to 20 mm Effective anchorage depth  $(h_{ef})$ : up to 240mm for non-cracked concrete



Retaining washer for overhead or horizontal installation Drill bit diameter (d<sub>0</sub>): 24 mm to 40 mm

Injection sytem VMU plus for concrete

Intended Use Cleaning and setting tools



1.	00	Drill with hammer drill a hole into the base material to the size and embedment depth required by the selected anchor (Table B1 or Table B2). In case of aborted drill hole, the drill hole shall be filled with mortar. Attention! Standing water in the bore hole must be removed before cleaning!
2a.	or	Cleaning with compressed air: Starting from the bottom or back of the bore hole, blow out the hole with compressed air (min. 6 bar) a minimum of four times. If the bore hole ground is not reached an extension must be used Manual cleaning: <u>Non cracked concrete:</u> drill bit diameter ≤ 20mm and effective anchorage depth ≤ 240mm <u>Cracked concrete:</u> M12, M16, ø 12, ø 14, ø 16 and effective anchorage depth ≤ 240mm Starting from the bottom or back of the bore hole, blow out the hole a minimum of four times. The blow-out pump can be used.
2b.		Check brush diameter (Table B3) and attach the brush to a drilling machine or a battery screwdriver. Brush the hole with an appropriate sized wire brush > d <sub>b,min</sub> (Table B3) a minimum of four times. If the bore hole ground is not reached with the brush, a brush extension must be used.
2c.	or	Cleaning with compressed air: Starting from the bottom or back of the bore hole, blow out the hole with compressed air (min. 6 bar) a minimum of four times. If the bore hole ground is not reached an extension must be used Manual cleaning: <u>Non cracked concrete</u> : drill bit diameter ≤ 20mm and effective anchorage depth ≤ 240mm <u>Cracked concrete</u> : M12, M16, Ø 12, Ø 14, Ø 16 and effective anchorage depth ≤ 240mm Starting from the bottom or back of the bore hole blow out the hole a minimum of four times. The blow-out pump can be used. After cleaning, the bore hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the bore hole. If necessary, the cleaning has to be repeated directly before dispensing the mortar. In-flowing water must not contaminate the bore hole again.
3.	A DESTRUCTION	Attach a supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool. Cut off the foil tube clip before use. For every working interruption longer than the recommended working time (Table B4 or Table B5) as well as for new cartridges, a new static-mixer shall be used.
4.	< her >	Prior to inserting the anchor rod into the filled bore hole, the position of the embedment depth shall be marked on the anchor rods.
5.	min3x +	Prior to dispensing into the drill hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour. For foil tube cartridges is must be discarded a minimum of six full strokes.
6.	+++ +	Starting from the bottom or back of the cleaned drill hole fill the hole up to approximately two- thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid creating air pockets. For embedment larger than 190 mm an extension nozzle must be used. For overhead and horizontal installation a retaining washer (Annex B3) and extension nozzle shall be used. Observe the gel-/ working times given in Table B4 or Table B5.
	27 8	

Intended Use Installation instructions



Inst	allation instruc	tions (continuation)	
7.		Push the threaded rod or reinforcing bar into the hole while turning slightly to en distribution of the adhesive until the embedment depth is reached.	sure proper
	× J	The anchor shall be free of dirt, grease, oil or other foreign material.	
8.		Make sure that the anchor is fully seated up to the full embedment depth and th visible at the top of the hole. If these requirements are not maintained, the appli renewed. For overhead installation the anchor should be fixed (e.g. wedges).	
9.	× ×	Allow the adhesive to cure to the specified time prior to applying any load or tor load the anchor until it is fully cured (attend Table B4 or Table B5).	que. Do not move or
10.		Remove excess mortar.	
11.		The fixture can be mounted after curing time. Apply installation torque T <sub>inst</sub> accousing a calibrated torque wrench.	rding to Table B1 by
Inje	ection sytem V	MU plus for concrete	
	nded Use allation instructions	Annex B5	



Concrete temperature	Maximum processing time	Minimum curing time in dry concrete <sup>1)</sup>
-10°C to -6°C	90 min <sup>2)</sup>	24 h <sup>2)</sup>
-5°C to -1°C	90 min	14 h
0°C to +4°C	45 min	7 h
+5°C to +9°C	25 min	2 h
+10°C to +19°C	15 min	80 min
+20°C to +29°C	6 min	45 min
+30°C to +34°C	4 min	25 min
+35°C to +39°C	2 min	20 min
+ 40°C	1,5 min	15 min
Cartridge temperature	+ 5°C t	o + 40°C

<sup>1)</sup> In wet concrete the curing time must be doubled.
 <sup>2)</sup> Cartridge temperature must be at min. + 15°C.

#### Maximum processing time and minimum curing time, VMU plus Polar Table B5:

Concrete temperature	Maximum processing time	Minimum curing time in dry concrete <sup>1)</sup>				
- 20°C to -16°C	75 min	24 h				
-15°C to -11°C	55 min	16 h				
-10°C to -6°C	35 min	10 h				
-5°C to -1°C	20 min	5 h				
0°C to +4°C	10 min	2,5 h				
+5°C to +9°C	6 min	80 min				
+10°C	6 min 60 min					
Cartridge temperature	- 20°C to	o + 10°C				

<sup>1)</sup> In wet concrete the curing time must be doubled.

Injection sytem VMU plus for concrete

Intended Use Processing time and curing time



$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Steel failure											
aracteristic bond resistance in cracked concrete C20/25         mperature range I: C/24°C       dry and wet concrete $\tau_{Rk,cr}$ [N/mm²]       4,0       5,0       5,5       5,5       5,5       6,5       6,5         mperature range II: C/50°C       dry and wet concrete $\tau_{Rk,cr}$ [N/mm²]       4,0       4,0       5,5       5,5       5,5       5,5       6,5       6,5         mperature range II: C/50°C       dry and wet concrete $\tau_{Rk,cr}$ [N/mm²]       2,5       3,0       4,0	Characteristic tension res	sistance	N <sub>Rk,s</sub>	[kN]				As	• f <sub>uk</sub>			
$\begin{array}{c c c c c c c c c } \mbox{mperature range I:} & \begin{tabular}{ c c c c c c c } \mbox{mperature range I:} & \begin{tabular}{ c c c c c c c } \mbox{mperature range II:} & \begin{tabular}{ c c c c c c c } \mbox{mperature range II:} & \begin{tabular}{ c c c c c c } \mbox{mperature range II:} & \begin{tabular}{ c c c c c } \mbox{mperature range II:} & \begin{tabular}{ c c c c c } \mbox{mperature range II:} & \begin{tabular}{ c c c c c } \mbox{mperature range II:} & \begin{tabular}{ c c c c c c } \mbox{diverses} & \begin{tabular}{ c c c c c c c } \mbox{diverses} & \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	Combined pull-out and	concrete cone f	ailure									
$\begin{array}{c c c c c c c c c c } \mbox{mperature range I:} & wet concrete & TRk.or & [N/mm^2] & 4,0 & 5,0 & 5,3 & 5,5 & 5,5 & not admissible \\ \hline \begin{tabular}{ c c c c c c c c c c c c c c c c c c c$	Characteristic bond resis	tance in cracked	concrete C2	20/25								
$\frac{1}{100 \text{ ded bore hole}} = \frac{1}{\text{trk,cr}} \left[ N/\text{mm}^2 \right] 4,0 4,0 5,5 5,5 5,5 \text{ not admissible} + \frac{1}{100 \text{ ded bore hole}} + \frac{1}{100  ded bore $	Temperature range I:		τ <sub>Rk,cr</sub>	<sub>:Rk,cr</sub> [N/mm <sup>2</sup> ] 4,0 5,0 5,5 5,5 5,5						5,5	6,5	6,5
$\begin{array}{c c c c c c c c } \mbox{mperature range II:} \\ C/50^{\circ}C \\ \hline \mbox{wet concrete} & $$T_{Rk,cr}$ $ [N/mm^2] $ 2,5 $ 3,5 $ 4,0 $ 4$	40°C/24°C		τ <sub>Rk,cr</sub>	[N/mm²]	4,0	4,0	5,5	5,5		not adn	nissible	
$\frac{\text{flooded}}{\text{bore hole}}  \begin{array}{c c c c c c } \hline T_{Rk,cr} & [N/mm^2] & 2,5 & 3,0 & 4,0 & 4,0 & \text{not admissible} \end{array}$ $\frac{\text{dry and}}{\text{wet concrete}}  \begin{array}{c c c c c c c } \hline T_{Rk,cr} & [N/mm^2] & 2,0 & 2,5 & 3,0 & 3,0 & 3,0 & 3,0 & 3,5 & 3,5 \end{array}$ $\frac{\text{dry and}}{\text{wet concrete}}  \begin{array}{c c c c c c } \hline T_{Rk,cr} & [N/mm^2] & 2,0 & 2,5 & 3,0 & 3,0 & 3,0 & 3,0 & 3,5 & 3,5 \end{array}$ $\frac{\text{flooded}}{\text{bore hole}}  \begin{array}{c c c c } \hline T_{Rk,cr} & [N/mm^2] & 2,0 & 2,5 & 3,0 & 3,0 & 3,0 & 3,0 & 3,5 & 3,5 \end{array}$ $reasing factor for \tau_{Rk,cr}  \begin{array}{c c c } \hline T_{Rk,cr} & [N/mm^2] & 2,0 & 2,5 & 3,0 & 3,0 & 3,0 & 3,0 & 3,5 & 3,5 \end{array}$ $\frac{\text{C25/30} & $-$1,02$ \\ \hline C30/37 & $-$1,04$ \\ \hline C40/50 & $-$1,08$ \\ \hline C45/55 & $-$1,09$ \\ \hline C50/60 & $-$1,10$ \\ \hline \text{ctor according to CEN/TS 1992-4-5 & k_8 & [-] & $-$7,2$ \\ \hline \text{ctor according to CEN/TS 1992-4-5 & k_cr & [-] & $-$7,2$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & k_cr & [-] & $-$7,2$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & k_cr & [-] & $-$7,2$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & k_cr & [-] & $-$7,2$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & k_cr & [-] & $-$7,2$ \\ \hline \text{al distance} & $s_{cr,N}$ & [mm] & $-$1,5 h_{ef}$ \\ \hline \text{al distance} & $s_{cr,N}$ & [mm] & $-$3,0 h_{ef}$ \\ \hline \text{tallation safety factor} & $\gamma_2 = $\gamma_{inst}$ & [-] & $1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $\gamma_2 = $\gamma_{inst}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline \text{cector according to CEN/TS 1992-4-5 & $k_{cr}$ & $-$1,0$ \\ \hline$	Temperature range II:		τ <sub>Rk,cr</sub>	[N/mm²]	2,5	3,5	4,0	4,0	4,0	4,0	4,5	4,5
$\begin{array}{c c c c c c c c } \hline \mbox{wet concrete} & $T_{Rk,cr}$ & $[N/mm^2]$ 2,0 & 2,5 & 3,0 & 3,0 & 3,0 & 3,0 & 3,0 & 3,0 & 3,5 & 3,5 \\ \hline \mbox{flooded} & $T_{Rk,cr}$ & $[N/mm^2]$ 2,0 & 2,5 & 3,0 & 3,0 & $not admissible$ \\ \hline \mbox{flooded} & $T_{Rk,cr}$ & $[N/mm^2]$ 2,0 & 2,5 & 3,0 & 3,0 & $not admissible$ \\ \hline \mbox{reasing factor for $T_{Rk,cr}$ & $I_102$ & $1,02$ & $1,04$ & $1,04$ & $1,04$ & $1,04$ & $1,07$ & $1,04$ & $1,04$ & $1,07$ & $1,04$ & $1,07$ & $1,04$ & $1,07$ & $1,04$ & $1,07$ & $1,04$ & $1,07$ & $1,04$ & $1,07$ & $1,09$ & $1,00$ $	80°C/50°C		τ <sub>Rk,cr</sub>	[N/mm²]	2,5	3,0	4,0	4,0		not adn	nissible	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Temperature range III:		τ <sub>Rk,cr</sub>	[N/mm²]	2,0	2,5	3,0	3,0	3,0	3,0	3,5	3,5
reasing factor for $\tau_{Rk,cr}$	120°C/72°C		τ <sub>Rk,cr</sub>	[N/mm²]	2,0	2,5	3,0	3,0		not adn	nissible	
ctor according to CEN/TS 1992-4-5k8[-]7,2ncrete cone failurector according to CEN/TS 1992-4-5kcr[-]7,2ge distance $C_{cr,N}$ [mm]1,5 hefal distance $S_{cr,N}$ [mm]3,0 heftallation safety factor y and wet concrete) $\gamma_2 = \gamma_{inst}$ [-]1,014not admissible	Increasing factor for $\tau_{Rk,cr}$		Ψc	C30/37 C35/45 C40/50 C45/55				1, 1, 1, 1,	04 07 08 09			
ncrete cone failurector according to CEN/TS 1992-4-5kcr[-]7,2ge distance $c_{cr,N}$ [mm]1,5 hefal distance $s_{cr,N}$ [mm]3,0 heftallation safety factor y and wet concrete) $\gamma_2 = \gamma_{inst}$ [-]1,0tallation safety factor $\gamma_2 = \gamma_{inst}$ [-]1,0tallation safety factor $\gamma_2 = \gamma_{inst}$ [-]1,4	Factor according to CEN	/TS 1992-4-5	k <sub>8</sub>									
ctor according to CEN/TS 1992-4-5k_{cr}[-]7,2ge distance $c_{cr,N}$ [mm]1,5 h_{ef}al distance $s_{cr,N}$ [mm]3,0 h_{ef}tallation safety factor y and wet concrete) $\gamma_2 = \gamma_{inst}$ [-]1,0tallation safety factor tallation safety factor $r_{acc} = r_{acc}$ 1.4									5			
ge distance $c_{cr,N}$ [mm]1,5 h_{ef}al distance $s_{cr,N}$ [mm] $3,0 h_{ef}$ tallation safety factor $\gamma_2 = \gamma_{inst}$ [-] $1,0$ y and wet concrete) $\gamma_2 = \gamma_{inst}$ [-] $1,0$ tallation safety factor $\gamma_2 = \gamma_{inst}$ [-] $1,0$		/TS 1992-4-5	kar	[-]				7	2			
al distance $s_{cr,N}$ [mm] $3,0 h_{ef}$ tallation safety factor $\gamma_2 = \gamma_{inst}$ [-] $1,0$ $1,2$ tallation safety factor $r_2 = r_1$ $r_2 = r_1$ $1,4$ not admissible												
tallation safety factor y and wet concrete) $\gamma_2 = \gamma_{inst}$ [-]1,01,2tallation safety factor $\nu = \nu$ [1]14not admissible	•											
tallation safety factor	Installation safety factor			1,0			- , -					
	Installation safety factor		$\gamma_2 = \gamma_{inst}$	[-]		1	,4			not adn	nissible	
	Concrete cone failure		$k_{cr}$ $C_{cr,N}$ $S_{cr,N}$ $\gamma_2 = \gamma_{inst}$	[-] [mm] [mm] [-]	1,0	1	,4	7 7 1,5	,2 ,2 5 h <sub>ef</sub> ) h <sub>ef</sub>	not adn	nissible	



	racteristic va c <mark>rete</mark>	lues for <b>t</b>	hreaded	rods	under	tensio	on loa	ds in i	non-cı	racked	1
Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
Steel failure											
Characteristic tension r	esistance	N <sub>Rk,s</sub>	[kN]				As	• f <sub>uk</sub>			
Combined pull-out an	d concrete cone	failure									
Characteristic bond res	istance in non-cr	acked concr	ete C20/25								
Temperature range I:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm²]	10	12	12	12	12	11	10	9
40°C/24°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm²]	7,5	8,5	8,5	8,5		not adr	nissible	
Temperature range II:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm²]	7,5	9	9	9	9	8,5	7,5	6,5
80°C/50°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm²]	5,5 6,5 6,5 6,5					not adr	nissible	
Temperature range III:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm²]	5,5	6,5	6,5	6,5	6,5	6,5	5,5	5,0
120°C/72°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm²]	4,0	5,0	5,0	5,0		not adr	nissible	
			C25/30				1,	02			
			C30/37				1,	04			
Increasing factor for $\tau_{Rk}$			C35/45				1,	07			
Increasing factor for t <sub>Rk</sub>	i,ucr	Ψc	C40/50	1,08							
			C45/55	1,09							
			C50/60	1,10							
Factor according to CE	N/TS 1992-4-5	k <sub>8</sub>	[-]				10	D,1			
Concrete cone failure											
Factor according to CE	N/TS 1992-4-5	k <sub>ucr</sub>	[-]				10	D,1			
Edge distance		C <sub>cr,N</sub>	[mm]	1,5 h <sub>ef</sub>							
Axial distance		S <sub>cr,N</sub>	[mm]				3,0	) h <sub>ef</sub>			
Splitting failure											
Edge distance for		C <sub>cr,sp</sub>	[mm]			1,0∙h <sub>ef</sub> ≤	≤ 2·h <sub>ef</sub> (2	$2,5-\frac{h}{h_{ef}}$	≤2,4·h <sub>ef</sub>		
Axial distance		S <sub>cr,sp</sub>	[mm]					cr,sp			
Installation safety factor (dry and wet concrete)		$\gamma_2 = \gamma_{inst}$	[-]	1,0				1,2			
Installation safety factor (flooded bore hole)	r	$\gamma_2 = \gamma_{inst}$	[-]		1	,4			not adr	nissible	
Injection sytem	VMU plus fo	or concre	ete								
<b>Performance</b> Characteristic values	s for <b>threaded</b> i	rods under	tension lo	oads in	non-ci	acked	concre	te	An	inex C	;2



## Table C3: Characteristic values for threaded rods under shear loads in cracked and noncracked concrete

Threaded rod			M8	M10	M12	M16	M20	M24	M27	M30
Steel failure without lever arm										
Characteristic shear resistance	V <sub>Rk,s</sub>	[kN]	[kN] 0,5 • A <sub>s</sub> • f <sub>uk</sub>							
Ductility factor according to CEN/TS 1992-4-5	k <sub>2</sub>	[-]				0	,8			
Steel failure with lever arm										
Characteristic bending moment	M <sup>0</sup> <sub>Rk,s</sub>	[Nm]				1,2 · V	V <sub>el</sub> ∙ f <sub>uk</sub>			
Concrete pry-out failure	1									
Factor k acc. to TR 029 or k₃ acc. to CEN/TS 1992-4-5	k <sub>(3)</sub>	[-]				2	,0			
Concrete edge failure										
Effective length of anchor	lf	[mm]	] I <sub>f</sub> = min(h <sub>ef</sub> ; 8 d <sub>nom</sub> )							
Outside diameter of anchor	d <sub>nom</sub>	[mm]	8	10	12	16	20	24	27	30
Installation safety factor	$\gamma_2 = \gamma_{inst}$	[-]				1	,0			

## Injection sytem VMU plus for concrete

#### Performance

Characteristic value for threaded rods under shear loads



Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30			
Tension load														
Steel failure														
Characteristic tension re	esistance	N <sub>Rk,s,seis</sub>	[kN]				As	• f <sub>uk</sub>						
Combined pull-out an	d concrete cone	failure												
Characteristic bond res	istance in concret	e C20/25 to	C50/60											
Temperature range I:	dry and wet concrete	T <sub>Rk,seis</sub>	[N/mm²]	2,5	3,1	3,7	3,7	3,7	3,8	4,5	4,5			
40°C/24°C	flooded bore hole	TRk, seis	[N/mm <sup>2</sup> ]	2,5	2,5	3,7	3,7		not adn	nissible				
Temperature range II:	dry and wet concrete	TRk,seis	[N/mm²]	1,6	2,2	2,7	2,7	2,7	2,8	3,1	3,1			
80°C/50°C	flooded bore hole	TRk,seis	[N/mm <sup>2</sup> ]	1,6	1,9	2,7	2,7		not adn	nissible				
Temperature range III:	dry and wet concrete	TRk,seis	[N/mm²]	1,3	1,6	2,0	2,0	2,0	2,1	2,4	2,4			
120°C/72°C	flooded bore hole	TRk,seis	[N/mm²]	1,3	1,6	2,0	2,0	-	not adn	nissible				
Increasing factor for T <sub>Rk</sub>		ψe	[-]				1	,0						
Installation safety factor (dry and wet concrete)	r	$\gamma_2 = \gamma_{inst}$	[-]	1,0				1,2						
(flooded bore hole)	r	$\gamma_2 = \gamma_{inst}$	[-]		1	4			not adn	nissible				
Shear load														
Steel failure without le	ever arm													
Characteristic shear res	sistance	V <sub>Rk,s,seis</sub>	[kN]				0,35 • /	A <sub>s</sub> ∙ f <sub>uk</sub>						
Steel failure with lever	r arm													
Characteristic bending	moment	M <sup>0</sup> Rk,s,seis	[Nm]		No	Perform	mance D	Determin	ed (NPI	))				



Rebar				Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Steel failure												
Characteristic tension re	esistance	N <sub>Rk,s</sub>	[kN]					A <sub>s</sub> ∙ f <sub>uk</sub> ¹	)			
Combined pull-out and	l concrete co	ne failure										
Characteristic bond resis	stance in crack	ed concre	te C20/25									
Temperature range I:	dry and wet concrete	τ <sub>Rk,cr</sub>	[N/mm²]	4,0	5,0	5,5	5,5	5,5	5,5	5,5	6,5	6,5
40°C/24°C	flooded bore hole	τ <sub>Rk,cr</sub>	[N/mm²]	4,0	4,0	5,5	5,5	5,5	not admissible			
Temperature range II:	dry and wet concrete	τ <sub>Rk,cr</sub>	[N/mm²]	2,5	3,5	4,0	4,0	4,0	4,0 4,0 4,5		4,5	
80°C/50°C	flooded bore hole	τ <sub>Rk,cr</sub>	[N/mm²]	2,5	3,0	4,0	4,0	4,0		not admissible		
Temperature range III:	dry and wet concrete	τ <sub>Rk,cr</sub>	[N/mm²]	2,0	2,5	3,0	3,0	3,0	3,0	3,0	3,5	3,5
120°C/72°C	flooded bore hole	τ <sub>Rk,cr</sub>	[N/mm²]	2,0	2,5	3,0	3,0	3,0		not adm	nissible	
Increasing factors for $\tau_{\text{RI}}$	k,cr	Ψc	C25/30 C30/37 C35/45 C40/50 C45/55 C50/60					1,02 1,04 1,07 1,08 1,09 1,10	4 7 8 9			
Factor acc. to CEN/TS 1	992-4-5	k <sub>8</sub>	[-]					7,2				
Concrete cone failure												
Factor acc. to CEN/TS 1	992-4-5	k <sub>cr</sub>	[-]					7,2				
Edge distance		C <sub>cr,N</sub>	[mm]					1,5 h <sub>ef</sub>				
Axial distance		S <sub>cr,N</sub>	[mm]					3,0 h <sub>ef</sub>				
Installation safety factor (dry and wet concrete)		$\gamma_2 = \gamma_{inst}$	[-]	1,0				1	,2			
Installation safety factor (flooded bore hole)		$\gamma_2 = \gamma_{inst}$	[-]	1,4 not admissible								

<sup>1)</sup>  $f_{uk} = f_{tk} = k \cdot f_{yk}$ 

Injection sytem VMU plus for concrete

#### Performance

Characteristic values for rebar under tension loads in cracked concrete



Rebar				Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
Steel failure													
Characteristic tension r	esistance	N <sub>Rk,s</sub>	[kN]					A <sub>s</sub> ∙ f <sub>uk</sub> ¹	)				
Combined pull-out an	d concrete con	e failure											
Characteristic bond res	istance in non-cr	acked con	crete C20/2	5									
Temperature range I:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm²]	10 12 12 12 12 12 11 10						10	8,5		
40°C/24°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm²]	7,5	8,5	8,5	8,5	8,5		not adr	nissible	9	
Temperature range II:	dry and wet concrete	TRk,ucr	[N/mm²]	7,5	9	9	9	9	9	8,0	7,0	6,0	
80°C/50°C	flooded bore hole	$\tau_{Rk,ucr}$	[N/mm²]	5,5	6,5	6,5	6,5	6,5		not adr	nissible	ole	
Temperature range III:	dry and wet concrete	τ <sub>Rk,ucr</sub>	[N/mm²]	5,5	6,5	6,5	6,5	6,5	6,5 6,5 6,0 5,0		5,0	4,5	
120°C/72°C	flooded bore hole	τ <sub>Rk,ucr</sub>	[N/mm²]	4,0	5,0	5,0	5,0	5,0		not adr	nissible	•	
hore noie noreasing factors for $\tau_{Rk,ucr}$		Ψc	C25/30 C30/37 C35/45 C40/50 C45/55					1,02 1,04 1,07 1,08 1,09					
Factor acc. to CEN/TS	1992-4-5	k <sub>8</sub>	C50/60 [-]					1,10 10,1					
Concrete cone failure													
Factor acc. to CEN/TS	1992-4-5	k <sub>ucr</sub>	[-]					10,1					
Edge distance		C <sub>cr,N</sub>	[mm]					1,5 h <sub>ef</sub>	6				
Axial distance		S <sub>cr,N</sub>	[mm]	3,0 h <sub>ef</sub>									
Splitting failure													
Edge distance for		C <sub>cr,sp</sub>	[mm]			1,0·h	<sub>ef</sub> ≤ 2 h	<sub>ef</sub> (2,5-	$\left(\frac{h}{h_{ef}}\right) \le 2$	2,4∙h <sub>ef</sub>			
Axial distance		S <sub>cr,sp</sub>	[mm]					2 C <sub>cr,sp</sub>					
Installation safety factor (dry and wet concrete)	r	$\gamma_2 = \gamma_{inst}$	[-]	1,0				1	,2				
	nstallation safety factor		[-]	1,4 not admissibl					nissible	•			
$f_{uk} = f_{tk} = k \cdot f_{yk}$													

Injection sytem VMU plus for concrete

#### Performance

Characteristic values for rebar under tension loads in non-cracked concrete



#### Table C7: Characteristic values for rebar under shear loads in cracked and non-cracked concrete Rebar Ø 8 Ø 10 Ø 12 Ø 14 Ø 16 Ø 20 Ø 25 Ø 28 Ø 32 Steel failure without lever arm $0,50 \cdot A_s \cdot f_{uk}^{(1)}$ Characteristic shear resistance [kN] V<sub>Rk,s</sub> Ductility factor according to [-] 0.8 k<sub>2</sub> CEN/TS 1992-4-5 Steel failure with lever arm $1,2 \cdot W_{el} \cdot f_{uk}^{(1)}$ Characteristic bending moment M<sup>0</sup><sub>Rk,s</sub> [Nm] Concrete pry-out failure Factor k acc. to TR 029 or 2.0 k<sub>(3)</sub> [-] k3 acc. to CEN/TS 1992-4-5 Concrete edge failure Effective length of anchor lf [mm] $l_f = min(h_{ef}; 8 d_{nom})$ 8 10 20 25 28 32 Outside diameter of anchor 12 16 dnom [mm] 14 Installation safety factor 1,0 [-] $\gamma_2 = \gamma_{inst}$ <sup>1)</sup> $\mathbf{f}_{uk} = \mathbf{f}_{tk} = \mathbf{k} \cdot \mathbf{f}_{yk}$

Injection sytem VMU plus for concrete

#### Performance

Characteristic values for rebar under shear loads in cracked and non-cracked concrete



Rebar				Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Tension load												
Steel failure												
Characteristic tension r	resistance	N <sub>Rk,s,seis</sub>	[kN]					As ∙ f <sub>uk</sub> ¹	()			
Combined pull-out an	id concrete con	e failure										
Characteristic bond res	istance in concr	ete C20/25	to C50/60	)								
Temperature range I:	dry and wet concrete	TRk,seis	[N/mm²]	2,5	3,1	3,7	3,7	3,7	3,7	3,8	4,5	4,5
40°C/24°C	flooded bore hole	TRk,seis	[N/mm²]	2,5	2,5	3,7	3,7	3,7	not admissible			
Temperature range II:	dry and wet concrete	TRk,seis	[N/mm²]	1,6	2,2	2,7	2,7	2,7	2,7	2,8	3,1	3,1
80°C/50°C	flooded bore hole	TRk,seis	[N/mm²]	1,6	1,9	2,7	2,7	2,7		not adr	nissible	
Temperature range III:	dry and wet concrete	TRk,seis	[N/mm²]	1,3	1,6	2,0	2,0	2,0	2,0	2,1	2,4	2,4
120°C/72°C	flooded bore hole	TRk, seis	[N/mm²]	1,3	1,6	2,0	2,0	2,0		not admissible		9
Increasing factor for $\tau_{RI}$	Ψc	[-]					1,0					
Installation safety facto	r	$\gamma_2 = \gamma_{inst}$	[-]	1,0				1	,2			-
(dry and wet concrete) Installation safety facto (flooded bore hole)	r	$\gamma_2 = \gamma_{inst}$	[-]			1,4		not admissible				,
Shear load												
Steel failure without le	ever arm			_								
Characteristic shear re	sistance	V <sub>Rk,s,seis</sub>	[kN]				0,3	5 • A <sub>s</sub> •	f <sub>uk</sub> 1)			
Steel failure with leve	r arm					5 m						
Characteristic bending	moment	M <sup>0</sup> Rk,s,seis	[Nm]			No Per	forman	ce Dete	ermined	(NPD)	1	
$f_{uk} = f_{tk} = k \cdot f_{yk}$												
njection sytem V												



Threaded rod			M8	M10	M12	M16	M20	M24	M27	M30	
Non-cracked concrete	C20/25										
Temperature range I:	$\delta_{N0}$ -factor	[mm/(N/mm²)]	0,021	0,023	0,026	0,031	0,036	0,041	0,045	0,049	
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,030	0,033	0,037	0,045	0,052	0,060	0,065	0,071	
Temperature range II:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119	
80°C/50°C	$\delta_{\text{N}\infty}\text{-factor}$	[mm/(N/mm²)]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172	
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm²)]	0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119	
120°C/72°C	$\delta_{\text{N}\infty}\text{-factor}$	[mm/(N/mm²)]	0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172	
Cracked concrete C20	/25										
Temperature range I:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,0	90	0,070						
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,1	05			0,1	05			
Temperature range II:	$\delta_{N0}$ -factor	[mm/(N/mm²)]	0,2	219			0,1	70			
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,2	255			0,2	245			
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm²)]	0,2	219			0,1	70			
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,2	255			0,2	245			

#### under tension loads<sup>1)</sup> (threaded red) . 00

<sup>1)</sup> Calculation of the displacement

 $\delta_{N0} = \delta_{N0}$ -Faktor  $\cdot \tau$ ;  $\tau$ : acting bond stress for tension load

 $\delta_{N\infty} = \delta_{N\infty}$ -Faktor  $\cdot \tau$ ;

#### Displacements under shear load<sup>1</sup> (threaded rod) Table C10:

Threaded rod	hreaded rod			M 10	M 12	M 16	M 20	M24	M 27	M 30
Non-cracked conc	rete C20/25									
All temperature	$\delta_{V0}$ -factor	[mm/(kN)]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V\infty}$ -factor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
Cracked concrete C20/25										
All temperature	$\delta_{V0}$ -factor	[mm/(kN)]	0,12	0,12	0,11	0,10	0,09	0,08	0,08	0,07
ranges	$\delta_{V\infty}$ -factor	[mm/(kN)]	0,18	0,18	0,17	0,15	0,14	0,13	0,12	0,10

Calculation of the displacement

 $\delta_{V0} = \delta_{V0}$ -factor · V; V: acting shear load

#### $\delta_{V\infty} = \delta_{V\infty}$ -factor · V;

## Injection sytem VMU plus for concrete

#### Performance

Displacements (threaded rod)



Table C11: Disp	lacemen	ts under ten	ision l	oad <sup>1)</sup>	(rebar	)					
Rebar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Non-cracked concrete	C20/25										
Temperature range I:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,021	0,023	0,026	0,028	0,031	0,036	0,043	0,047	0,052
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,030	0,033	0,037	0,041	0,045	0,052	0,061	0,071	0,075
Temperature range II:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,126
80°C/50°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,181
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,126
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,181
Cracked concrete C20	/25										
Temperature range I:	$\delta_{N0}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,0	90				0,070			
40°C/24°C	$\delta_{N\infty}$ -factor	[mm/(N/mm <sup>2</sup> )]	0,1	05				0,105			
Temperature range II:	$\delta_{N0}$ -factor	[mm/(N/mm²)]	0,2	219				0,170			
80°C/50°C	$\delta_{N_{\infty}}$ -factor	[mm/(N/mm²)]	0,2	255				0,245			
Temperature range III:	$\delta_{N0}$ -factor	[mm/(N/mm²)]	0,2	219				0,170			
120°C/72°C	$\delta_{N\infty}$ -factor	[mm/(N/mm²)]	0,2	255				0,245			

<sup>1)</sup> Calculation of the displacement

 $\begin{array}{ll} \delta_{\text{N0}} = \delta_{\text{N0}}\text{-}\text{Faktor} \ \cdot \ \tau; & \tau: \text{ acting bond stress for tension load} \\ \delta_{\text{N}\infty} = \delta_{\text{N}\infty}\text{-}\text{Faktor} \ \cdot \ \tau; & \end{array}$ 

## Table C12: Displacements under shear load<sup>1)</sup> (rebar)

Table C12. Displacements under siteal load (rebai)											
Rebar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Non-cracked cond	crete C20/25										
All temperature	$\delta_{V0}$ -factor	[mm/(kN)]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03
ranges	$\delta_{V_\infty}$ -factor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,04	0,04
Cracked concrete C20/25											
All temperature	$\delta_{V0}$ -factor	[mm/(kN)]	0,12	0,12	0,11	0,11	0,10	0,09	0,08	0,07	0,06
ranges	$\delta_{V\infty}$ -factor	[mm/(kN)]	0,18	0,18	0,17	0,16	0,15	0,14	0,12	0,11	0,10
<sup>1)</sup> Calculation of the displacement $\delta_{V0} = \delta_{V0}$ -factor · V; V: acting shear load $\delta_{V\infty} = \delta_{V\infty}$ -factor · V;											
Injection syte									$\neg$		
<b>Performance</b> Displacements (r	ebar)								4	Annex	C10